

EMO

Date:	May 20, 2020
To:	City of Dublin
From:	Matt Stechschulte, PE
Subject:	The Corners Stormwater Management Plan
Copies:	

This memo summarizes the stormwater management approach for the Corners project located at the northwest corner of Rings Road and Frantz Road. The proposed project was analyzed under the Dublin Smart Parking Lot Stormwater Management Plan (SWMP) dated May 19, 2017. The Dublin Smart Parking Lot report accounted for the Corners project area within Subarea 03 which discharges to Wet Basin 01. Wet Basin 01 is interconnected with Wet Basin 02 before discharging east across Frantz Road. Subarea 03 was to be developed at 75% impervious cover per the Smart Parking Lot SWMP. The proposed project was calculated to be 66.8% impervious which is less than what was assumed. Due to the proposed project containing less impervious cover than what was assumed in the Dublin Smart Parking Lot SWMP the existing BMPs (Wet Basins 01 & 02) are able to adequately proposed quantity and quality control for the proposed development without the need for any modifications. Below Table 1 & 2 summarize the differences between the Dublin Smart Parking Lot proposed release rates and basin elevations and what the actual release rates 'and elevations will be based on The Corners project actual impervious cover.

Planned vs. Actual Release Rates							
Storm Event (yr.)	Subarea 01 and 03 Allowable Release Rates (cfs.)	Subarea 02 Allowable Release Rates (cfs.)	Offsite Release Rates (cfs.)	Total Allowable Release Rates (cfs.)	Proposed Release Rates (cfs.)		
1	16.70	4.97	10.74	32.42	10. 17 9.56		
2	16.70	4.97	11.88	33.56	13.30 12.85		
5	16.70	4.97	13.65	35.33	16. 62 16.41		
10	16.70	4.97	15.14	36.82	18. 18 18.00		
25	16.70	4.97	17.23	38.91	20.12 19.98		
50	63.37	4.97	19.73	88.07	21.37 21.27		
100	81.46	24.87	23.25	129.59	22.54 22.44		

Table 1

Plannea VS. Actual basin Performance Summary					
Storm Event (yr.)	Wet Basins 01 and 02 Inflow Rates (cfs.)	Maximum W.S.E., T.O.B. = 867.00 (feet)	Storage Volume Utilized (ac-ft)		
1	73.67 71.57	863.76 863.73	2.742 2.690		
2	91.18 88.93	863.95 863.91	3.078 3.002		
5	115.62 113.48	864.38 864.32	3.863 3.737		
10	135.13 133.11	864.86 864.81	4.778 4.658		
25	162.21 160.43	865.52 865.47	6.098 5.975		
50	185.00 183.27	866.04 866.00	7.209 7.095		
100	209.66 207.87	866.55 866.52	8.352 8.255		

Table 2 Planned vs. Actual Basin Performance Summary

Wet Basins 01 & 02 Detention Storage Utilized: 8.352 8.255 ac-ft (100-year storm event) Wet Basins 01 & 02 Detention Storage Provided: 9.388 ac-ft

Due to the reduction to impervious cover the water quality volume also reduces from what was previously assumed in the Dublin Smart Parking Lot SWMP. Table 3 below summarizes the difference between the previously planned water quality calculations and actual water quality calculations based on The Corners proposed site conditions.

Planned vs. Actual Water Quality Calculations					
		Water Quality	Water Quality		
	Tributary area	Volume	Elevation		
Basin Identifier	(acres)	(ac-ft)	(feet)		
Wet Basins 01 & 02	55.502	1.354 1.323	862.92 862.90		

Table 3

Due to The Corners project proposing a reduction to impervious cover from what was previously accounted for in the Dublin Smart Parking Lot SWMP that existing wet basins (Wet Basin 01 & Wet Basin 02) are able to provide adequate quantity and quality control for the proposed site.

Project Name: Dublin Smart Parking Lot

Water Quality Volume Calculation

<u>Wet Basins 01 & 02</u>	
Area =	26.159 acres
% imp =	0.69
C =	0.49
WQv =	0.799 ac-ft
<u>Offsite</u>	
Area =	29.343 acres
% imp =	0.73
C =	0.53
WQv =	0.965 ac-ft
75% of WQv=	1.323 ac-ft
(for wet basins)	
WQv Elevation=	862.90 feet

Water quality volume calculated using the Ohio EPA formula

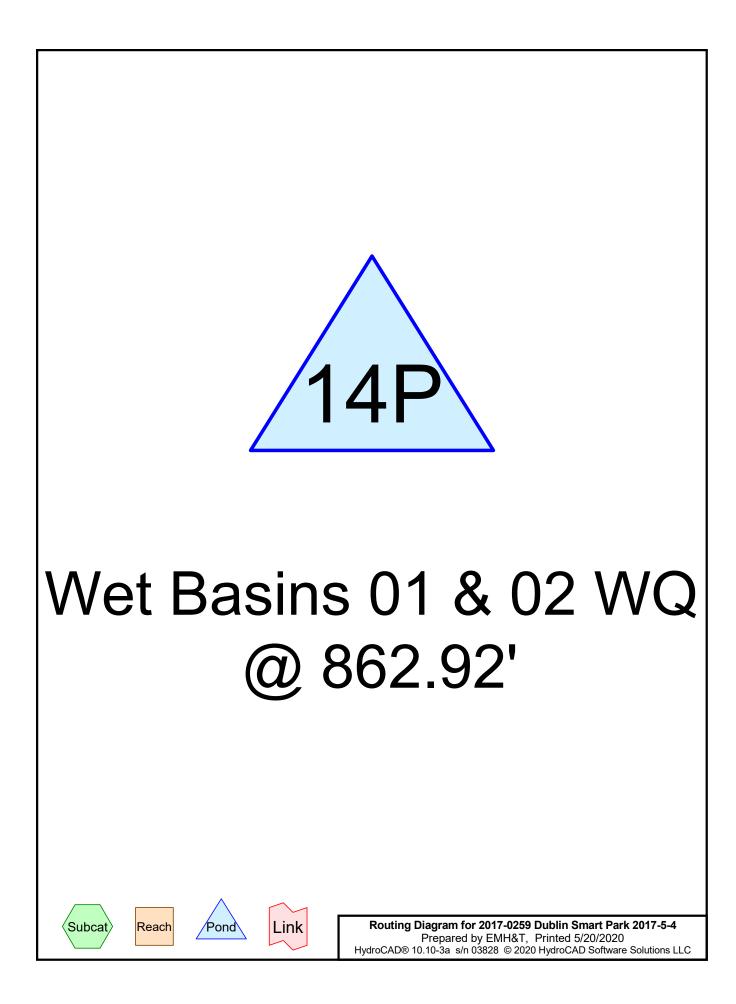
<u>Ohio EPA formula</u>

$$WQv = \frac{C \times P \times A}{12}$$

 $\begin{array}{l} A = area \; (acres) \\ P = 0.75'' \\ C = runoff \; coefficient \; (calculated using the ASCE method) \end{array}$

 $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

Where i = fraction of post-construction impervious surface



Printed 5/20/2020 Page 2

 Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
 1	1-year	Type II 24-hr		Default	24.00	1	2.20	2
2	2-year	Type II 24-hr		Default	24.00	1	2.63	2
3	5-year	Type II 24-hr		Default	24.00	1	3.24	2
4	10-year	Type II 24-hr		Default	24.00	1	3.74	2
5	25-year	Type II 24-hr		Default	24.00	1	4.44	2
6	50-year	Type II 24-hr		Default	24.00	1	5.02	2
7	100-year	Type II 24-hr		Default	24.00	1	5.63	2

Rainfall Events Listing

Summary for Pond 14P: Wet Basins 01 & 02 WQ @ 862.92'

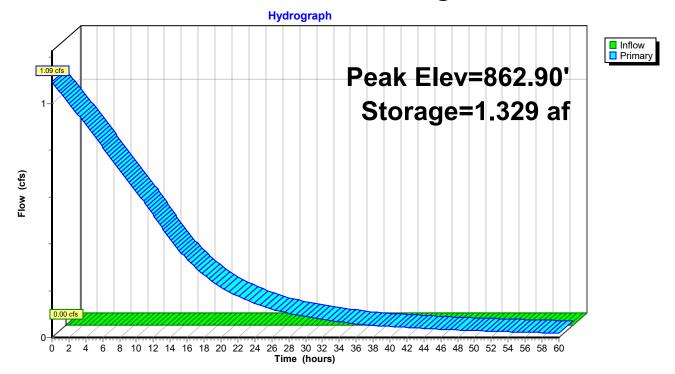
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af
Outflow	=	1.09 cfs @	0.00 hrs, Volume=	1.252 af, Atten= 0%, Lag= 0.0 min
Primary	=	1.09 cfs @	0.00 hrs, Volume=	1.252 af

Routing by Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Starting Elev= 862.90' Surf.Area= 1.560 ac Storage= 1.329 af Peak Elev= 862.90' @ 0.00 hrs Surf.Area= 1.560 ac Storage= 1.329 af

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert Av	vail.Storage	e Storage Description
#1	862.00'	5.548 af	f Wet Basin 01 (Prismatic) Listed below (Recalc)
#2	862.00'	3.834 af	
		9.382 af	
			C C
Elevatior	n Surf.Area	Inc.St	Store Cum.Store
(feet) (acres)	(acre-fe	feet) (acre-feet)
862.00	0.827	0.	0.000 0.000
863.00	0.937	0.	0.882 0.882
864.00		0.).993 1.875
865.00			.107 2.983
866.00			.224 4.207
866.50			0.656 4.862
867.00) 1.401	0.	0.686 5.548
- 1			
Elevation		Inc.St	
(feet	· · /	(acre-fe	
862.00			0.000 0.000
863.00	0.642	0.	0.604 0.604
864.00	0.720	0.).681 1.285
865.00	0.801	0.	0.760 2.045
866.00	0.884	0.).843 2.888
866.50	0.950	0.4).459 3.347
867.00	0.998	0.4	0.487 3.834
Device	Routing	Invert Ou	Outlet Devices
#1	Primary	862.00' 5.0	.0" Vert. WQ orifice X 2.00 C= 0.600
		Lir	imited to weir flow at low heads

Primary OutFlow Max=1.09 cfs @ 0.00 hrs HW=862.90' (Free Discharge) ←1=WQ orifice (Orifice Controls 1.09 cfs @ 4.00 fps)



Pond 14P: Wet Basins 01 & 02 WQ @ 862.92'

Hydrograph for Pond 14P: Wet Basins 01 & 02 WQ @ 862.92'

Time	Inflow	Storage	Elevation	Primary
(hours)	(cfs)	(acre-feet)	(feet)	(cfs)
0.00	0.00	1.329	862.90	1.09
2.00	0.00	1.156	862.79	1.00
4.00	0.00	0.999	862.69	0.91
6.00	0.00	0.856	862.59	0.81
8.00	0.00	0.730	862.51	0.72
10.00	0.00	0.619	862.43	0.62
12.00	0.00	0.524	862.37	0.52
14.00	0.00	0.447	862.31	0.42
16.00	0.00	0.385	862.27	0.33
18.00	0.00	0.335	862.24	0.27
20.00	0.00	0.296	862.21	0.21
22.00	0.00	0.264	862.19	0.18
24.00	0.00	0.237	862.17	0.15
26.00	0.00	0.215	862.15	0.12
28.00	0.00	0.197	862.14	0.10
30.00	0.00	0.181	862.13	0.09
32.00	0.00	0.167	862.12	0.08
34.00	0.00	0.155	862.11	0.07
36.00	0.00	0.145	862.10	0.06
38.00	0.00	0.136	862.10	0.05
40.00	0.00	0.127	862.09	0.05
42.00	0.00	0.120	862.09	0.04
44.00	0.00	0.113	862.08	0.04
46.00	0.00	0.107	862.08	0.04
48.00	0.00	0.102	862.07	0.03
50.00	0.00	0.097	862.07	0.03
52.00	0.00	0.092	862.07	0.03
54.00	0.00	0.088	862.06	0.02
56.00	0.00	0.084	862.06	0.02
58.00	0.00	0.080	862.06	0.02
60.00	0.00	0.077	862.06	0.02

Events for Pond 14P: Wet Basins 01 & 02 WQ @ 862.92'

Event	Inflow	Primary	Elevation	Storage
	(cfs)	(cfs)	(feet)	(acre-feet)
1-year	0.00	1.09	862.90	1.329

TABLE OF CONTENTS

Project Reports

- 1 Routing Diagram
- 2 Rainfall Events Listing

1-year Event

3 Pond 14P: Wet Basins 01 & 02 WQ @ 862.92'

Multi-Event Tables

6 Pond 14P: Wet Basins 01 & 02 WQ @ 862.92'

