Engineers, Surveyors, Planners, Scientists

## EMO

| Date: | May 20, 2020 |
| :--- | :--- |
| To: | City of Dublin |
| From: | Matt Stechschulte, PE |
| Subject: | The Corners Stormwater Management Plan |

Copies:

This memo summarizes the stormwater management approach for the Corners project located at the northwest corner of Rings Road and Frantz Road. The proposed project was analyzed under the Dublin Smart Parking Lot Stormwater Management Plan (SWMP) dated May 19, 2017. The Dublin Smart Parking Lot report accounted for the Corners project area within Subarea 03 which discharges to Wet Basin 01. Wet Basin 01 is interconnected with Wet Basin 02 before discharging east across Frantz Road. Subarea 03 was to be developed at $75 \%$ impervious cover per the Smart Parking Lot SWMP. The proposed project was calculated to be $66.8 \%$ impervious which is less than what was assumed. Due to the proposed project containing less impervious cover than what was assumed in the Dublin Smart Parking Lot SWMP the existing BMPs (Wet Basins $01 \& 02$ ) are able to adequately proposed quantity and quality control for the proposed development without the need for any modifications. Below Table $1 \& 2$ summarize the differences between the Dublin Smart Parking Lot proposed release rates and basin elevations and what the actual release rates 'and elevations will be based on The Corners project actual impervious cover.

Table 1
Planned vs. Actual Release Rates
$\left.\begin{array}{|c||c|c|c|c|cc|}\hline \text { Storm Event } \\ \text { (yr.) }\end{array} \begin{array}{c}\text { Subarea 01 } \\ \text { and 03 } \\ \text { Allowable } \\ \text { Release Rates } \\ \text { (cfs.) }\end{array} \quad \begin{array}{c}\text { Subarea 02 } \\ \text { Allowable } \\ \text { Release Rates } \\ \text { (cfs.) }\end{array} \quad \begin{array}{c}\text { Offsite } \\ \text { Release Rates } \\ \text { (cfs.) }\end{array} \begin{array}{c}\text { Total Allowable } \\ \text { Release Rates } \\ \text { (cfs.) }\end{array} \quad \begin{array}{c}\text { Proposed } \\ \text { Release Rates } \\ \text { (cfs.) }\end{array}\right]$

Table 2
Planned vs. Actual Basin Performance Summary

| Storm <br> Event <br> (yr.) | Wet Basins O1 and 02 <br> Inflow Rates <br> (cfs.) | Maximum W.S.E., T.O.B. <br> $=867.00$ <br> (feet) | Storage Volume <br> Utilized <br> (ac-ft) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | $73.67-71.57$ | 863.76 | 863.73 | 2.742 | 2.690 |
| 2 | $91.18-88.93$ | $863.95-863.91$ | 3.078 | 3.002 |  |
| 5 | 115.62 | 113.48 | $864.38-864.32$ | 3.863 | 3.737 |
| 10 | $135.13-133.11$ | $864.86-864.81$ | 4.778 | 4.658 |  |
| 25 | 162.21 | 160.43 | 865.52 | 865.47 | 6.098 |
| 50 | 185.00 | 183.27 | 866.04 | 866.00 | 7.209 |
| 100 | 209.66 | 207.87 | $866.55-866.52$ | 8.352 | 8.255 |

Wet Basins 01 \& 02 Detention Storage Utilized: 8.3528 .255 ac-ft (100-year storm event) Wet Basins 01 \& 02 Detention Storage Provided: 9.388 ac-ft

Due to the reduction to impervious cover the water quality volume also reduces from what was previously assumed in the Dublin Smart Parking Lot SWMP. Table 3 below summarizes the difference between the previously planned water quality calculations and actual water quality calculations based on The Corners proposed site conditions.

Table 3
Planned vs. Actual Water Quality Calculations

| Basin Identifier | Tributary area <br> (acres) | Water Quality <br> Volume <br> (ac-ft) | Water Quality <br> Elevation <br> (feet) |
| :---: | :---: | :---: | :---: |
| Wet Basins 01 \& 02 | 55.502 | $1.354-1.323$ | 862.92862 .90 |

Due to The Corners project proposing a reduction to impervious cover from what was previously accounted for in the Dublin Smart Parking Lot SWMP that existing wet basins (Wet Basin 01 \& Wet Basin 02) are able to provide adequate quantity and quality control for the proposed site.

Project Name: Dublin Smart Parking Lot

## Water Quality Volume Calculation

Wet Basins 01 \& 02

| Area $=$ | 26.159 acres |
| :--- | :---: |
| $\%$ imp $=$ | 0.69 |
| $C=$ | 0.49 |
| $W Q v=$ | 0.799 ac-ft |

Offsite
Area $=$
\% imp =
$\mathrm{C}=$
$W Q v=$
29.343 acres
0.73
0.53
$75 \%$ of WQv=
0.965 ac-ft
(for wet basins)
WQv Elevation=
862.90 feet

Water quality volume calculated using the Ohio EPA formula
Ohio EPA formula
$W Q v=\frac{C \times P \times A}{12}$
$A=$ area (acres)
P = 0.75"
$C=$ runoff coefficient (calculated using the ASCE method)
$C=0.858 i^{3}-0.78 i^{2}+0.774 i+0.04$
Where $\mathrm{i}=$ fraction of post-construction impervious surface


## Wet Basins 01 \& 02 WQ

 @ 862.92'

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2017-0259 Dublin Smart Park 2017-5-4
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## Rainfall Events Listing

| Event\# | Event <br> Name | Storm Type | Curve | Mode | Duration <br> (hours) | B/B | Depth <br> (inches) |
| ---: | :--- | :--- | :--- | ---: | ---: | ---: | ---: |
| AMC |  |  |  |  |  |  |  |
| 1 | 1-year | Type II 24-hr | Default | 24.00 | 1 | 2.20 | 2 |
| 2 | 2-year | Type II 24-hr | Default | 24.00 | 1 | 2.63 | 2 |
| 3 | 5-year | Type II 24-hr | Default | 24.00 | 1 | 3.24 | 2 |
| 4 | 10-year | Type II 24-hr | Default | 24.00 | 1 | 3.74 | 2 |
| 5 | 25-year | Type II 24-hr | Default | 24.00 | 1 | 4.44 | 2 |
| 6 | 50-year | Type II 24-hr | Default | 24.00 | 1 | 5.02 | 2 |
| 7 | 100-year | Type II 24-hr | Default | 24.00 | 1 | 5.63 | 2 |

## Summary for Pond 14P: Wet Basins 01 \& 02 WQ @ 862.92'

| Inflow | $=$ | $0.00 \mathrm{cfs} @$ | 0.00 hrs, Volume $=$ |
| :--- | :--- | :--- | :--- |
| Outflow | $=$ | 0.000 af |  |
| Primary | $=$ | $1.09 \mathrm{cfs} @$ | 0.00 hrs, Volume $=$ |
| 1.252 af, , Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |  |  |  |
|  | 0.00 hrs, Volume $=$ | 1.252 af |  |

Routing by Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs
Starting Elev=862.90' Surf.Area=1.560 ac Storage=1.329 af
Peak Elev= 862.90' @ 0.00 hrs Surf.Area= 1.560 ac Storage= 1.329 af
Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time $=$ (not calculated: no inflow)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | :--- |
| $\# 1$ | $862.00^{\prime}$ | 5.548 af | Wet Basin 01 (Prismatic) Listed below (Recalc) |
| $\# 2$ | $862.00^{\prime}$ | 3.834 af | Wet Basin 02 (Prismatic) Listed below (Recalc) |
|  |  | 9.382 af | Total Available Storage |


| Elevation <br> (feet) | Surf.Area <br> (acres) | Inc.Store <br> (acre-feet) | Cum.Store <br> (acre-feet) |
| ---: | ---: | ---: | ---: |
| 862.00 | 0.827 | 0.000 | 0.000 |
| 863.00 | 0.937 | 0.882 | 0.882 |
| 864.00 | 1.050 | 0.993 | 1.875 |
| 865.00 | 1.165 | 1.107 | 2.983 |
| 866.00 | 1.282 | 1.224 | 4.207 |
| 866.50 | 1.342 | 0.656 | 4.862 |
| 867.00 | 1.401 | 0.686 | 5.548 |


| Elevation <br> (feet) | Surf.Area <br> (acres) | Inc.Store <br> (acre-feet) | Cum.Store <br> (acre-feet) |
| ---: | ---: | ---: | ---: |
| 862.00 | 0.566 | 0.000 | 0.000 |
| 863.00 | 0.642 | 0.604 | 0.604 |
| 864.00 | 0.720 | 0.681 | 1.285 |
| 865.00 | 0.801 | 0.760 | 2.045 |
| 866.00 | 0.884 | 0.843 | 2.888 |
| 866.50 | 0.950 | 0.459 | 3.347 |
| 867.00 | 0.998 | 0.487 | 3.834 |


| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | 862.00 | 5.0" Vert. WQ orifice X 2.00 <br> Limited to weir flow at low heads | $C=0.600$ |

Primary OutFlow Max=1.09 cfs @ 0.00 hrs HW=862.90' (Free Discharge)
$L_{1=W Q}$ orifice (Orifice Controls 1.09 cfs @ 4.00 fps )

Pond 14P: Wet Basins 01 \& 02 WQ @ 862.92'
Hydrograph


Hydrograph for Pond 14P: Wet Basins 01 \& 02 WQ @ 862.92'

| Time <br> (hours) | Inflow <br> (cfs) | Storage <br> (acre-feet) | Elevation <br> (feet) | Primary <br> (cfs) |
| ---: | ---: | ---: | ---: | ---: |
| 0.00 | $\mathbf{0 . 0 0}$ | 1.329 | 862.90 | 1.09 |
| 2.00 | 0.00 | 1.156 | 862.79 | 1.00 |
| 4.00 | 0.00 | 0.999 | 862.69 | 0.91 |
| 6.00 | 0.00 | 0.856 | 862.59 | 0.81 |
| 8.00 | 0.00 | 0.730 | 862.51 | 0.72 |
| 10.00 | 0.00 | 0.619 | 862.43 | 0.62 |
| 12.00 | 0.00 | 0.524 | 862.37 | 0.52 |
| 14.00 | 0.00 | 0.447 | 862.31 | 0.42 |
| 16.00 | 0.00 | 0.385 | 862.27 | 0.33 |
| 18.00 | 0.00 | 0.335 | 862.24 | 0.27 |
| 20.00 | 0.00 | 0.296 | 862.21 | 0.21 |
| 22.00 | 0.00 | 0.264 | 862.19 | 0.18 |
| 24.00 | 0.00 | 0.237 | 862.17 | 0.15 |
| 26.00 | 0.00 | 0.215 | 862.15 | 0.12 |
| 28.00 | 0.00 | 0.197 | 862.14 | 0.10 |
| 30.00 | 0.00 | 0.181 | 862.13 | 0.09 |
| 32.00 | 0.00 | 0.167 | 862.12 | 0.08 |
| 34.00 | 0.00 | 0.155 | 862.11 | 0.07 |
| 36.00 | 0.00 | 0.145 | 862.10 | 0.06 |
| 38.00 | 0.00 | 0.136 | 862.10 | 0.05 |
| 40.00 | 0.00 | 0.127 | 862.09 | 0.05 |
| 42.00 | 0.00 | 0.120 | 862.09 | 0.04 |
| 44.00 | 0.00 | 0.113 | 862.08 | 0.04 |
| 46.00 | 0.00 | 0.107 | 862.08 | 0.04 |
| 48.00 | 0.00 | 0.102 | 862.07 | 0.03 |
| 50.00 | 0.00 | 0.097 | 862.07 | 0.03 |
| 52.00 | 0.00 | 0.092 | 862.07 | 0.03 |
| 54.00 | 0.00 | 0.088 | 862.06 | 0.02 |
| 56.00 | 0.00 | 0.084 | 862.06 | 0.02 |
| 58.00 | 0.00 | 0.080 | 862.06 | 0.02 |
| 60.00 | 0.00 | 0.077 | 862.06 | 0.02 |

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2017-0259 Dublin Smart Park 2017-5-4
Multi-Event Tables
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## Events for Pond 14P: Wet Basins 01 \& 02 WQ @ 862.92'

| Event | Inflow <br> (cfs) | Primary <br> (cfs) | Elevation <br> (feet) | Storage <br> (acre-feet) |
| :--- | ---: | ---: | ---: | ---: |
| 1 -year | $\mathbf{0 . 0 0}$ | $\mathbf{1 . 0 9}$ | $\mathbf{8 6 2 . 9 0}$ | $\mathbf{1 . 3 2 9}$ |

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2017-0259 Dublin Smart Park 2017-5-4
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