ABBREVIATIONS

010000-0002 (08/22/19)
AB	ANCHOR BOLT
	ADJACENT
AFF	ABOVE FINISH FLOOR
ALT	ALTERNATE
ARCH	ARCHITECT(URAL)
BLDG	BUILDING
BLK	BLOCKING
BLW	BELOW
BM	BEAM
BN	BOUNDARY NAILING
B.O.	BOTTOM OF
BRR	BUCKLING-RESTRAINED BRACE
BRG	BEARING
BS	BOTH SIDES
BTWN	BETWEEN
C	
CIP	CAST IN PLACE
CJ	CONTROL/CONSTRUCTION JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLG	CEILING
CNIU	COLUMN
	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CP	COMPLETE PENETRATION
D⊳	BAR OR BOIT DIAMETER
DBL	DOUBLE
DEMO	DEMOLITION
DET	DETAIL
	DIAMETER
	DIMENSION
DIR	DIRECTION
DO	DITTO
DWG	DRAWING
(E)	EXISTING
EA EE	
FJ	EXPANSION JOINT
EMBED	EMBEDMENT
ELEC	ELECTRICAL
ELEV	ELEVATION OR ELEVATOR
E.U. FOR	
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EACH SIDE OR EDGE SCREW
EW	EACH WAY
EXP FYT	EXPANSION
FIN	FINISH
FLG	FLANGE
FLR	FLOOR
FN	FIELD NAILING
F.U. FS	FACE OF FAR SIDE OR FIELD SCREW
FRMG	FRAMING
FRP	FIBER REINFORCED POLYMER
FT	FOOT OR FEET
FTG	
G GA	GAGE
GALV	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GLB	GLUED-LAMINATED BEAM
nad HD	
HDR	HEADER
HGR	HANGER
HK	HOOK
HORIZ	HORIZONTAL
HC HC	HIGH YUINI HIGH STRENGTH
HSB	HIGH STRENGTH BOI T
HSS	HOLLOW STRUCTURAL SECTION
HT	HEIGHT
ID	INSIDE DIAMETER

IN INT	INCH
IOR	INSPECTOR OF RECORD
JST	JOIST
VI F	JUINT KIPS PER LINEAR FOOT
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	
LFRS	LATERAL FORCE RESISTING
	SYSTEM
LLH	
LLV	I ONGITUDINAI
LP	LOW POINT
LS	LAP SPLICE
LWC	
MB	MACHINE BOLT
MECH	MECHANICAL
MFR	MANUFACTURER
MIIN	
(N)	NEW
NA	NEUTRAL AXIS
NS	NEAR SIDE OR NON-SHRINK
NWC	NORMAL WEIGHT CONCRETE
OC	ON CENTER
OD	OUTSIDE DIAMETER
O.F. OH	
OPNG	OPENING
PDF	POWDER/POWER DRIVEN
ы	FASTENER
PJ PJP	PARTIAL JOINT PENETRATION
PL	PLATE
PLC(S)	PLACE(S)
	POUNDS PER LINEAR FOUT
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
рт	
PT	PRESSURE TREATED OR POST TENSION
PT QTY	PRESSURE TREATED OR POST TENSION QUANTITY
PT QTY RAD, R	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS
PT QTY RAD, R REF REINE	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING
PT QTY RAD, R REF REINF REQD	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED
PT QTY RAD, R REF REINF REQD (S)	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR
PT QTY RAD, R REF REINF REQD (S) SB	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT
PT QTY RAD, R REF REINF REQD (S) SB SC	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL
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PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SI AB ON GRADE
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE
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PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ SS STD STGRD STIFF STL	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE STAINLESS STEEL STANDARD STAGGERED STIFFENER STEEL
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PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ SS STD STGRD STIFF STL STRUCT T&B THK	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE STAINLESS STEEL STANDARD STAGGERED STIFFENER STEEL STRUCTURAL TOP & BOTTOM THICK
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ SS STD STGRD STIFF STL STRUCT T&B THK THRD	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE STAINLESS STEEL STANDARD STAGGERED STIFFENER STEEL STRUCTURAL TOP & BOTTOM THICK THREADED
PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ SS STD STIFF STL STRUCT T&B THK THRD T.O. TDAIO	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE STAINLESS STEEL STANDARD STAGGERED STIFFENER STEEL STRUCTURAL TOP & BOTTOM THICK THREADED TOP OF
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PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ SS STD STGRD STIFF STL STRUCT T&B THK THRD T.O. TRANS TYP UNO VERT VIF W/	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE STAINLESS STEEL STANDARD STAGGERED STIFFENER STEEL STRUCTURAL TOP & BOTTOM THICK THREADED TOP OF TRANSVERSE TYPICAL UNLESS NOTED OTHERWISE VERTICAL VERIFY IN FIELD WITH
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PT QTY RAD, R REF REINF REQD (S) SB SC SCHED SEOR SHTG SIM SMS SN SOG SQ SS STD STGRD STIFF STL STRUCT T&B THK THRD T.O. TRANS TYP UNO VERT VIF W/ W/O WF, W WD WO WP	PRESSURE TREATED OR POST TENSION QUANTITY RADIUS REFERENCE REINFORCING REQUIRED "SIMPSON" STRONG TIE CO. OR "USP" W/ EQUIVALENT ICC VALUES SILL BOLT SAW CUT OR SLIP CRITICAL SCHEDULE STRUCTURAL ENGINEER OF RECORD SHEATHING SIMILAR SHEET METAL SCREW SILL NAIL SLAB ON GRADE SQUARE STAINLESS STEEL STANDARD STAGGERED STIFFENER STEEL STRUCTURAL TOP & BOTTOM THICK THREADED TOP OF TRANSVERSE TYPICAL UNLESS NOTED OTHERWISE VERTICAL VERIFY IN FIELD WITH WITHOUT WIDE FLANGE WELDED WHERE OCCURS WORK POINT

WWF WELDED WIRE FABRIC

STRUCTURAL STEEL

SPECIFICATIONS AND STANDARD OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC), AS CONTAINED IN THE LATEST EDITION OF "AISC MANUAL OF STEEL CONSTRUCTIO					
ALL STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO LINE. TEMPORARY BRACING SHALL BE INSTALLED AND SHALL BE LEFT IN PLACE UNTIL OTHER MEANS IS PROVIDED TO ADEQUATELY BRACE THE STRUCTURE.					
PROVIDE THE FOLLOWING MATERIALS FOR STRUCTURAL STEEL UNO:					
SHAPE	MATERIAL / GRADE				
WIDE FLANGE SECTIONS & TEES	ASTM A992				
PLATES, ANGLES, CHANNELS	ASTM A36				
SQUARE OR RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS)	ASTM A500, GRADE C (Fy = 50 KSI) OR ASTM A1085				
ROUND HOLLOW STRUCTURAL SECTIONS (HSS)	ASTM A500, GRADE C (Fy = 46 KSI) OR ASTM A1085				
PIPES	ASTM A53 TYPE E OR S, GRADE B (Fy = 35 KSI)				
MACHINE BOLTS (MB)	ASTM A307				
HIGH STRENGTH BOLTS (HSB)	ASTM A325 TYPE N				
WELDED HEADED STUDS	ASTM A108				
THREADED RODS FOR ANCHOR BOLTS	ASTM F1554, GRADE 55				
HIGH STRENGTH PLATE	ASTM A572, GRADE 50				
 a. EXCEPT AS OTHERWISE NOTED, ALL BOLTS SHA b. WHERE WELDING TO GRADE 55 THREADED ANC 	ALL BE HIGH STRENGTH BOLTS.				

- ASTM F1554 GRADE 55 WITH SUPPLEMENT S1.
- PRIOR TO FABRICATION.
- OR EQUIVALENT, AND WELDED (IN ACCORDANCE WITH MANUFACTURER'S CAPACITY OF THE CONNECTOR.
- 6. BOLTS WITH UPSET THREADS ARE NOT ALLOWED. USE THE APPROPRIATE NUT AND WASHER TYPE FOR THE SPECIFIED BOLT.
- 7. ALL STEEL FABRICATION SHALL BE PERFORMED BY A LICENSED FABRICATOR.
- OF STANDARD PRACTICE, SECTION 10.
- SHOWN IN THESE DRAWINGS.
- 11. DO NOT CUT HOLES IN STRUCTURAL STEEL WITHOUT APPROVAL OF THE SEOR.
- BEFORE APPLYING LOADS
- 13. ALL WORK SHALL BE IN CONFORMANCE WITH ANY AND ALL TESTING, INSPECTION, COORDINATION OF QUALITY CONTROL AND QUALITY ASSURANCE IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.
- 14. USE HOT-DIP GALVANIZED BOLTS IN ACCORDANCE WITH ASTM A-123.

1. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE

4. ALL CONNECTIONS NOT SHOWN SHALL CONFORM TO THE "AISC MANUAL OF STEEL CONSTRUCTION" AND SHALL BE SUBMITTED ON SHOP DRAWINGS FOR REVIEW BY EOR

5. ALL WELDED HEADED STUDS, THREADED STUDS, AND DEFORMED BARS SHALL BE NELSON, RECOMMENDATIONS BY CERTIFIED WELDERS) SO AS TO FULLY DEVELOP THE TENSILE

8. ALL STRUCTURAL STEEL AND MISCELLANEOUS STEEL PERMANENTLY EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED AFTER FABRICATION UNLESS A WEATHER PROOF COATING IS SPECIFIED BY THE ARCHITECT, UNO. STAINLESS AND WEATHERING STEELS, WHERE SPECIFIED, ARE EXEMPT FROM THIS REQUIREMENT. GALVANIZED SURFACES SHALL BE PROTECTED DURING CONSTRUCTION AND SHALL BE REPAIRED AS NECESSARY. ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL COMPLY WITH AISC CODE

9. SEE ARCHITECTURAL DRAWINGS FOR NAILER HOLES, WELDED STUDS OR OTHER ITEMS NOT

10. WHERE STEEL IS EMBEDDED IN CONCRETE OR MASONRY, PROVIDE HOLES AS REQUIRED FOR PASSAGE OF CONTINUOUS REINFORCING BARS WHERE INDICATED ON DRAWINGS.

12. PLACE NON-SHRINK OR DRYPACK GROUT UNDER ALL BASE PLATES AND ALLOW TO CURE

QUALIFICATION, AND QUALITY ASSURANCE PROVISIONS AS REQUIRED BY THE BUILDING CODE AND ANY APPLICABLE STANDARDS. THESE STANDARDS INCLUDE, BUT ARE NOT LIMITED TO THE LATEST VERSION OF THE FOLLOWING: AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"; AISC 341 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS", AWS D1.1 "STRUCTURAL WELDING CODE - STEEL"; AWS D1.8 "STRUCTURAL WELDING CODE - SEISMIC SUPPLEMENT"; AND RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS". CONFORMANCE TO SUPPLEMENTS TO THESE STANDARDS, IF PUBLISHED ON OR BEFORE THE DATE OF PERMIT ISSUANCE, IS ALSO REQUIRED. ALTHOUGH THESE CONTRACT DOCUMENTS INCLUDE GENERAL REFERENCES TO CODES AND STANDARDS, AND REFERENCES TO OR INCLUSIONS OF SPECIFIED PROVISIONS, OMISSIONS OF ANY APPLICABLE CODE, STANDARD, OR PROVISION DOES NOT RELIEVE THE GENERAL CONTRACTOR FROM COMPLIANCE TO THE APPLICABLE REQUIREMENTS.

STRUCTURAL CONCRETE

- CONCRETE SHALL BE MIXED, PLACED AND CURED IN ACCORDANCE WITH ACI 318 AND ACI 301 LATEST EDITION, AND PROJECT SPECIFICATIONS.
- CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL (AS IN WALLS) SO AS TO CAUSE SEGREGATION OF AGGREGATES. IN SUCH CASES, HOPPERS AND VERTICAL CHUTES OR TRUNKS SHALL BE USED. CHUTES OR TRUNKS SHALL BE OF VARIABLE LENGTHS SO THAT FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED SIX FEET. A SUFFICIENT NUMBER OF CHUTES OR TRUNKS SHALL BE USED TO ENSURE THE CONCRETE IS KEPT LEVEL AT ALL TIMES.
- 3. CONSTRUCTION JOINTS SHALL BE CLEANED AND ROUGHENED BY REMOVING THE ENTIRE SURFACE TO EXPOSE CLEAN AGGREGATE SOLIDLY EMBEDDED IN THE MORTAR MATRIX. SEE PLANS AND DETAILS FOR LOCATION AND TYPE OF CONSTRUCTION JOINT. LOCATIONS OF ADDITIONAL CONSTRUCTION JOINTS NOT SHOWN ON THESE PLANS SHALL BE SUBMITTED FOR APPROVAL TO THE EOR PRIOR TO PLACING ANY CONCRETE.
- 4. STRUCTURAL CONCRETE SHALL MEET THE FOLLOWING DESIGN CRITERIA

LOCATION	MIN 28-DAY COMP STRENGTH	CONC TYPE	MAX AGGREGATE SIZE	MAX W/C RATIO
FOUNDATION	3000 PSI	NWC	1 1/2"	0.55

- a. MAXIMUM AIR DRY UNIT WEIGHT OF LIGHTWEIGHT CONCRETE SHALL NOT EXCEED 110 PCF, UNLESS APPROVED BY EOR.
- b. WHEN THE USE OF PLASTICIZER (ASTM C1017, TYPE I OR II) OR WATER REDUCER (ASTM C494, TYPE F OR G) IS USED, MAXIMUM SLUMP SHALL BE 4" PRIOR TO ADMIXTURE AND 8" INCLUDING ADMIXTURE AT THE POINT OF DELIVERY. IN THE ABSENCE OF PLASTICIZER AND WATER REDUCER, SLUMP AT THE POINT OF DELIVERY SHALL NOT FXCFFD 4".
- W/C RATIO INDICATES WATER TO CEMENTITIOUS MATERIALS RATIO.
- FOR INTERIOR SLABS ON GRADE AND ALL OTHER SLABS RECEIVING ADHERED FLOORING FINISHES (I.E., GLUED, ETC.), THE MAXIMUM W/C RATIO SHALL NOT EXCEED 0.46. CURING COMPOUNDS USED ON CONCRETE THAT IS TO RECEIVE FINISHES SHALL BE COMPATIBLE WITH TILE AND ADHESIVES OR GROUTS IN ACCORDANCE WITH MANUFACTURER'S DATA AND BE APPROVED BEFORE USE.
- e. SLABS ON GRADE, TOPPING SLABS, AND ELEVATED CONCRETE FLOORS SHALL HAVE A MAXIMUM SHRINKAGE RATE OF 0.04% AT 28 DAYS PER ASTM C 157 (CURING TEST SPECIMENS TO BE CONSISTENT WITH FIELD CONDITIONS), OR USING EMBEDDED VIBRATING WIRE STRAIN GAUGES. RESULTS OF TESTING SHALL BE SUBMITTED TO ENGINEER.
- SEE ACI 318 FOR ADDITIONAL REQUIREMENTS REGARDING MAXIMUM AGGREGATE SIZE.AGGREGATE GRADATION OF 3/8" MAXIMUM (PEA GRAVEL) SHALL NOT BE USED WHERE FINISHED CONCRETE SURFACE IS EXPOSED TO VIEW.
- 5. CONCRETE MIX DESIGN AND TESTING SHALL MEET THE REQUIREMENTS OF THE BUILDING CODE, AND SPECIFICATIONS. ALL CONCRETE MIXES SHALL BE DESIGNED PER ACI 318 SECTION 5.2 BY A RECOGNIZED TESTING LAB STAMPED AND SIGNED BY A LICENSED CALIFORNIA CIVIL ENGINEER AND SUBMITTED TO THE EOR FOR REVIEW PRIOR TO CONCRETE PLACEMENT. STRUCTURAL CONCRETE MIXES SHALL CONSIST OF 5 SACK MINIMUM UNO
- 6. AGGREGATES IN NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C-33 (HARDROCK). AGGREGATES IN LIGHT WEIGHT CONCRETE SHALL CONFORM TO ASTM C-330.
- 7. COMPRESSIVE STRENGTH TEST REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE EOR.
- 8. PORTLAND CEMENT SHALL BE TYPE II AND SHALL CONFORM TO ASTM C150, LOW ALKALI. MILL TESTS WITH CERTIFICATES OF COMPLIANCE SHALL BE SUBMITTED.
- 9. FLY ASH OR OTHER POZZOLANS CONFORMING TO ASTM C618 CLASS F MAY BE USED AS A PARTIAL SUBSTITUTION FOR PORTLAND CEMENT UP TO A MAXIMUM OF 25% TOTAL CEMENTITIOUS MATERIALS BY WEIGHT IF THE MIX DESIGN IS PROPORTIONED BY FIELD EXPERIENCE OR TRIAL MIXTURES.
- 10. CONCRETE MIXING OPERATIONS, ETC. SHALL CONFORM TO ASTM C94.
- 11. LEAN CONCRETE, WHERE SPECIFICALLY INDICATED, SHALL CONTAIN 2 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE.
- 12. DRYPACK OR NONSHRINK GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5000 PSI, AND CONSIST OF MASTERFLOW 713, EUCON NS GROUT, SIKA GROUT 212, OR APPROVED EQUAL. FOR THICK GROUT LAYERS FOLLOW MANUFACTURER'S GUIDELINES TO ATTAIN THE REQUIRED STRENGTH, WHICH MAY INCLUDE THE ADDITION OF PEA GRAVEL. FOR BASE PLATES LARGER THAN 6 SQUARE FEET, USE HI-FLOW GROUT OR MASTERFLOW 928.
- 13. DO NOT USE ANY CONCRETE OR GROUT CONTAINING CHLORIDES. WATER USED IN MIX SHALL BE CLEAN AND POTABLE.
- 14. PRIOR TO ERECTING ANY ELEMENTS THAT LOAD THE FOUNDATION, CONCRETE MUST REACH AN UNCONFINED COMPRESSION STRENGTH OF 2000 PSI MINIMUM AS DETERMINED BY TESTING OR PREVIOUSLY DOCUMENTED DATA FOR THE MIX DESIGN USED UNDER SIMILAR CONDITIONS, AND MUST BE ALLOWED TO CURE FOR A MINIMUM OF 3 DAYS.
- 15. MAINTAIN CONCRETE ABOVE 50 DEGREES FAHRENHEIT AND IN A MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT UNLESS OTHERWISE ACCEPTED BY EOR.
- 16. SEE ARCHITECTURAL DRAWINGS FOR WALL OPENINGS, WALL OFFSETS, CHAMFERS, KERFS, DRIPS AND FOR EXTENT OF DEPRESSIONS, RAMPS, ETC.
- 17. PROVIDE SLEEVES FOR ALL PIPES THROUGH CONCRETE WALLS AND FOOTINGS WHERE SHOWN ON THESE DRAWINGS. CORING IS NOT PERMITTED WITHOUT PRIOR APPROVAL BY THE FOR.
- 18. EXPOSED CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH 3/4" CHAMFER OR 1/2" RADIUS TOOLED EDGE, UNO.
- 19. ALL FOOTINGS SHALL BEAR ON FIRM UNDISTURBED RESIDUAL SOIL AND/OR ENGINEERED EARTH FILL COMPACTED TO 95% OF ITS MAXIMUM DRY DENSITY AS PER ASTM D698 (STANDARD PROCTOR) UNLESS NOTED OTHERWISE. A LICENSED GEOTECHNICAL ENGINEER SHALL CONFIRM SOIL CAPACITY PRIOR TO CONCRETE PLACEMENT.
- 20. TOP OF PIERS SHALL BE SLOPED SUCH THAT MOISTURE CANNOT ACCUMULATE.
- 21. THE MINIMUM CONCRETE COVER FOR THE PROTECTION OF REINFORCEMENT SHALL BE AS NOTED.

REINFORCING STEEL

REINFORCING GRADES FOR CONCRETE OR MASONRY: ALL BARS UNLESS NOTED OTHERWISE... b. TIES AND STIRRUPS .. WELDED WIRE FABRIC . d. ALL BARS TO BE WELDED ...

NOTE: ALL BARS SHALL BE DEFORMED.

MAINTAIN CONCRETE COVER FROM FACE OF CONCRETE TO EDGE OF ALL REINFORCEMENT AS FOLLOWS (UNO):

ONDITION	COVER	
AST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"	
ORMED AND EXPOSED TO EARTH OR WEATHER		
- #6 BARS AND LARGER	2"	
- #5 BARS AND SMALLER	1 1/2"	
ROVIDE THE LARGEST COVER REQUIRED FOR ALL APPLICABLE CO	ONDITIONS. WHE	RE
TIRRUPS OR TIES ARE USED. ENSURE THAT THE COVER FOR LON	GITUDINAL BARS	SIS

- REINFORCEMENT SHALL BE PLACED IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE". EACH REINFORCING BAR SHALL BE WIRED TO A CROSS BAR AT A MAXIMUM SPACING OF 24"OC. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING IN POSITIONS SHOWN ON THE PLANS. DO NOT USE WOOD OR BRICK TO SUPPORT REINFORCING.
- 4. SPLICES IN CONTINUOUS REINFORCEMENT AS USED IN WALLS, WALL FOOTINGS, ETC., SHALL HAVE A CLASS "B" LAP (1'-6" MIN) AND THE SPLICES IN ADJACENT BARS SHALL BE NOT LESS THAN 5'-0" APART. VERTICAL WALL BARS SHALL BE SPLICED AT OR NEAR FLOOR LINES. BARS MAY BE WIRED TOGETHER AT SPLICES OR LAPS EXCEPT FOR TOP REINFORCING OF BEAMS AND SLABS OR WHERE SPECIFICALLY DETAILED TO BE SEPARATED. WELDED WIRE FABRIC SHALL BE LAPPED 12" MINIMUM.
- ALL DOWELS, ANCHOR BOLTS AND OTHER HARDWARE TO BE SET IN CONCRETE SHALL BE TIED IN PLACE PRIOR TO PLACEMENT OF CONCRETE. NO WET SETTING, STABBING, RODDING OR OTHER MOVEMENT OF EMBEDDED ITEMS SHALL BE PERFORMED DURING PLACEMENT OF CONCRETE
- 6. BEND REINFORCING BARS COLD.

ADEQUATE.

- STEEL SHALL BE KEPT CLEAN AND FREE OF RUST.
- 8. DOWELS BETWEEN FOOTING AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING AS THE MAIN REINFORCING UNO.
- 9. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN PLACE INSPECTION IS MADE.
- 10. CHAIRS OR SPACERS FOR REINFORCING SHALL BE PLASTIC WHEN RESTING ON EXPOSED SURFACES.
- 11. WHERE LONGITUDINAL REINFORCING BARS ARE PLACED IN 2 OR MORE LAYERS, BARS IN THE UPPER LAYERS SHALL BE PLACED DIRECTLY ABOVE BARS IN THE BOTTOM LAYER.
- 12. ALL BENDS WITHIN STIRRUPS, HOOPS, AND CROSS-TIES SHALL ENGAGE A LONGITUDINAL BAR. PROVIDE #4 SPACER BAR WHERE A LONGITUDINAL BAR IS NOT SPECIFICALLY DFTAILED
- 13. WELDING OF REINFORCING BARS SHALL BE PERFORMED PER AMERICAN WELDING SOCIETY (AWS) D1.4 USING E90XX ELECTRODES FOR A615 REINFORCING AND E80XX ELECTRODES FOR A706 REINFORCING.

....ASTM A615 OR A706, GRADE 60 ...ASTM A615 OR A706, GRADE 60

...ASTM A1064 .. ASTM A706, GRADE 60

E #3 RUPS OR HES ARE USED, ENSURE THAT THE COVER FOR LONGITUDINAL BARS IS

GENERAL 0000-0000 (08/22/18)

- 1. REFER TO THE TYPICAL DETAIL SHEETS FOR TYPICAL DETAILS OF CONSTRUCTION. TYPICAL DETAILS APPLY TO ALL CONSTRUCTION UNLESS SPECIFICALLY NOTED OR SHOWN OTHERWISE. WHERE CONDITIONS REQUIRE MODIFICATIONS OF A TYPICAL DETAIL, THE CONTRACTOR SHALL SUBMIT MODIFIED DETAIL FOR APPROVAL BY THE ENGINEER OF RECORD PRIOR TO FABRICATION AND INSTALLATION. DETAILS OF CONSTRUCTION NOT SHOWN SHALL BE OF SAME NATURE AS THOSE SHOWN FOR SIMILAR CONSTRUCTION
- 2. CONTRACTOR SHALL CONSIDER THE PROJECT SPECIFICATIONS A PART OF THE CONTRACT DOCUMENTS. WHERE INFORMATION IS CONFLICTING, SPECIFIC DETAILS SHALL GOVERN OVER TYPICAL DETAILS WHICH SHALL GOVERN OVER THESE NOTES WHICH SHALL GOVERN OVER SPECIFICATIONS.
- 3. ALL DIMENSIONS ON STRUCTURAL DRAWINGS SHALL BE CHECKED AGAINST ARCHITECTURAL DIMENSIONS. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE OMITTED OR NOT CLEAR, CONTACT THE ARCHITECT (ARCH) OR ENGINEER OF RECORD (EOR). ALL DIMENSIONS RELATED TO EXISTING CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR. DIMENSIONS ARE TO THE FACE OF STUDS, AND TO CENTERLINE OF COLUMNS UNO.
- 4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IMMEDIATELY NOTIFY THE EOR OF ANY CONFLICTS BETWEEN THE STRUCTURAL DRAWINGS AND OTHER DRAWINGS; OR EXISTING CONDITIONS NOT SHOWN OR DIFFERENT FROM THOSE SHOWN ON DRAWINGS PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE SCOPE THAT IS IN CONFLICT UNTIL THE CONFLICT IS RESOLVED WITH THE AFFECTED PARTIES.
- 5. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE SHOWN THEY DO NOT INDICATE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE CONSTRUCTION AND ALL ADJACENT PROPERTIES DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT ARE NOT LIMITED TO BRACING, SHORING OF LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT OR EOR SHALL NOT INCLUDE OBSERVATION OF THE ABOVE ITEMS.
- SUBSTITUTION REQUESTS FOR MATERIALS SPECIFIED ON THE STRUCTURAL DRAWINGS MAY BE CONSIDERED WITH MATERIALS HAVING EQUIVALENT OR GREATER CAPACITY AND PERFORMANCE. CURRENT EVALUATION REPORTS AND PRODUCT INFORMATION SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER DEMONSTRATING THE REQUIRED CAPACITY AND PERFORMANCE OF THE MATERIAL TO BE SUBSTITUTED. WRITTEN APPROVAL FROM THE EOR SHALL BE OBTAINED PRIOR TO THE SUBSTITUTION OF ANY MATERIAL SPECIFIED ON THE STRUCTURAL DOCUMENTS.
- 7. ALL WORK IS NEW (N) UNLESS INDICATED AS EXISTING (E).
- 8. CONSTRUCTION MATERIALS SHALL BE DISTRIBUTED WHEN PLACED ON THE STRUCTURE SUCH THAT LOADS DO NOT EXCEED DESIGN LIVE LOADS OR RESULT IN AN UNBALANCED CONDITION.
- REFER TO THE PROJECT SPECIFICATIONS FOR SHOP DRAWING REQUIREMENTS AND SUBMITTALS. REVIEW OF SHOP DRAWINGS AND SUBMITTALS BY THE EOR IS FOR GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR WILL REMAIN RESPONSIBLE FOR ALL ERRORS OF DETAILING, FABRICATION, AND FOR CORRECT FITTING OF ALL STRUCTURAL MEMBERS, INCLUDING COORDINATION WITH OTHER TRADES. SHOP DRAWINGS AND SUBMITTALS DO NOT CONSTITUTE CHANGE ORDERS. ANY PROPOSED CHANGES TO THE STRUCTURAL DOCUMENTS MUST BE SUBMITTED IN WRITING AS A REQUEST FOR SUBSTITUTION TO THE ARCHITECT AND EOR FOR APPROVAL.
- 10. CORE DRILLS REQUIRED SHALL NOT CUT ANY REINFORCING. THE CONTRACTOR IS TO COORDINATE WORK OF ALL TRADES TO ENSURE COMPLIANCE. ALL CORE DRILLS ARE TO BE PRESENTED TO THE INSPECTOR OF RECORD (IOR) FOR VERIFICATION. THE IOR IS TO DOCUMENT CORES EXAMINED INDICATING AN ABSENCE OF REINFORCING.
- 11. STRUCTURAL JOINT DIMENSIONS SHOWN ON PLANS (EXPANSION, SEISMIC, SEPARATION, ETC) (WHERE OCCURS) INDICATE THE MINIMUM CLEAR DISTANCE REQUIRED. SEE PLANS, DETAILS, AND ARCHITECTURAL DRAWINGS FOR ADDITIONAL REQUIREMENTS.

STRUCTURAL DESIGN CRITERIA

- ALL NEW WORK SHALL BE IN CONFORMANCE WITH THE INTERNATIONAL BUILDING CODE (IBC) 2018 EDITION (TITLE 24, PART 2), INCLUDING ALL AMENDMENTS. ALL STANDARDS USED SHALL BE THE LATEST VERSION APPROVED BY THE CODE ENFORCEMENT AGENCY ON THE DATE OF THE PERMIT ISSUANCE UNLESS SPECIFICALLY NOTED OTHERWISE. THE PURPOSE OF THIS CODE IS TO, IN PART, ESTABLISH THE MINIMUM REQUIREMENTS TO SAFEGUARD THE PUBLIC HEALTH. SAFETY AND GENERAL WELFARE THROUGH STRUCTURAL STRENGTH AND STABILITY. STRUCTURES DESIGNED IN ACCORDANCE WITH THE CODE ARE LIKELY TO HAVE A LOW PROBABILITY OF COLLAPSE BUT MAY SUFFER SERIOUS STRUCTURAL AND NON-STRUCTURAL DAMAGE IF SUBJECTED TO THE DESIGN EARTHQUAKE.
- 2. WIND DESIGN INFORMATION:
- RISK CATEGORY II EXPOSURE C BASIC WIND SPEED (3 SEC GUST), V_{ult} = 120 MPH
- 3. SEISMIC DESIGN INFORMATION: I_e = 1.0 RISK CATEGORY II DESIGN CAT. = E SITE CLASS D $S_S = 1.949$ $S_1 = 0.77$ $S_{DS} = 1.559$ $S_{D1} = 0.924$



miyamoto.	1450 Halyard Drive, Suite One T: (916) 373-1995 West Sacramento, CA 95691 miyamotointernational.com	DT FOR INSTALL
Tim Hortons.	ANYWHERE USA	DESIGN SET - NO
REV RELEAS	E	DATE 00/00/00
PROJECT NO. DRAWN BY: CHECKED BY: APPROVED BY: SHEET TITLE: TY	PICAL NOTES	MI2001051 AJR KB SMK
SHEET NUMBER:	S0.01	



1" = 1'-0"

